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2. AMENDMENT/MODIFICATION NO.	3. EFFECTIVE DATE	4. REQUISITION/PURCHASE REQ. NO.		1	5. PROJE	CT NO.(Ifapplicable)		
A0003	24-Apr-2018							
6. ISSUED BY CODE	W912EP	7. ADMINISTERED BY (Ifother than item 6)		CO	DE			
USA ENGINEER DISTRICT, JACKSONVILLE CONTRACTING DIVISION 701 SAN MARCO BLVD JACKSONVILLE FL 32207-8175		See Item 6						
8. NAME AND ADDRESS OF CONTRACTOR (	No., Street, County, S	State and Zip Code)	Х	9A. AMENDM	ENT OF S	SOLICITATION NO.		
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(a) By completing Items 8 and 15, and returning or (c) By separate letter or telegram which includes a re RECEIVED AT THE PLACE DESIGNATED FOR TH REJECTION OF YOUR OFFER. If by virtue of this am	copies of the amendmen ference to the solicitation a E RECEIPT OF OFFERS I rendment you desire to chan	t; (b) By acknowledging receipt of this amendment amendment numbers. FAILURE OF YOUR APRIOR TO THE HOUR AND DATE SPECIFIED age an offer already submitted, such change may be a continued to the specifical continued to the speci	ent on ACK DMA	n each copy of the of NOWLEDGMENT AY RESULT IN ade by telegramor le	ТО ВЕ	d;		
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A. THIS CHANGE ORDER IS ISSUED PURSU CONTRACT ORDER NO. IN ITEM 10A.	ANT TO: (Specify a	uthority) THE CHANGES SET FORTH	IN	ITEM 14 ARE N	MADE IN	THE		
office, appropriation date, etc.) SET FORT	H IN ITEM 14, PURS	SUANT TO THE AUTHORITY OF FA			as change	s in paying		
C. THIS SUPPLEMENTAL AGREEMENT IS	ENTERED INTO PU	RSUANT TO AUTHORITY OF:						
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E. IMPORTANT: Contractor is not,	is required to sign	n this document and return	co	pies to the issuin	g office.			
	CATION (Organized	by UCF section headings, including solic	itat	ion/contract subj	ect matte	r		
Herbert Hoover Dike Rehabilitation, Structure R	Replacement S-288 (F	IP-1) Reconstruction						
See Continuation Sheet for complete description	on of changes.							
The Proposal due date is changed from to 27 A	April 2018 @ 2:00 PM	to 04 May 2018 @ 2:00 PM.						
All other conditions remain unchanged.								
Except as provided herein, all terms and conditions of the do	cument referenced in Items	PA or 10A, as heretofore changed, remains uncha	nged	and in full force and	effect.			
15A. NAME AND TITLE OF SIGNER (Type or	print)	16A. NAME AND TITLE OF CO	NT	RACT ING OFFI	CER (Typ	pe or print)		
		TEL:		EMAIL:				
15B. CONTRACTOR/OFFEROR	15C. DATE SIGNEI		RIC.		]	16C. DATE SIGNED		
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# SF 30 CONTINUATION SHEET

Herbert Hoover Dike Rehabilitation, Structure Replacement S-288 (HP-1) Reconstruction

Amendment 0003

#### SUMMARY OF CHANGES

#### 1. SPECIFICATIONS:

The text changes have been updated with additions noted by underlined text and deletions noted by line/cross-outs, and pertain only to changes made by this amendment. The entire section is replaced if there is any change. The Project Table of Contents and Submittal Register are replaced without underlines and cross-outs if there are changes to these documents.

#### Changes to Specifications:

# Volume 1 of 2 - Technical Specifications:

N/A

#### Volume 2 of 2 - Technical Specifications:

DELETE Section 35 42 00 and REPLACE with the attached revised Section 35 42 00.

**DELETE** Submittal Register in Section 01 33 00 and **REPLACE** with the attached revised Submittal Register.

#### 2. DRAWINGS:

DELETE Drawing No. S-63 and REPLACE with the attached revised Drawing No. S-63.

(End of Summary of Changes)

W912EP-18-R-0013 Amendment 0003

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# DIVISION 35 - WATERWAY AND MARINE CONSTRUCTION

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  - 1.2.1 Soil-Bentonite Fill 1.2.2 Embankment Fill

  - 1.2.3 Select Fill
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- -- End of Section Table of Contents --

# SECTION 35 42 00

# SOIL-BENTONITE FILL

# PART 1 GENERAL

# 1.1 REFERENCES

The publications listed below form a part of this specification to the extent referenced. The publications are referred to within the text by the basic designation only.

# AMERICAN PETROLEUM INSTITUTE (API)

API Spec 13A	(2010;	Errata	1	2014;	Errata	2-3	2015)
	Specif	ication	fo	r Dril	lling-Fl	luid	Materials

# ASTM INTERNATIONAL (ASTM)

ASTM D1556	(2007) Density and Unit Weight of Soil in Place by the Sand-Cone Method
ASTM D1557	(2012; E 2015) Standard Test Methods for Laboratory Compaction Characteristics of Soil Using Modified Effort (56,000 ft-lbf/ft3) (2700 kN-m/m3)
ASTM D2216	(2010) Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass
ASTM D2487	(2011) Soils for Engineering Purposes (Unified Soil Classification System)
ASTM D422	(1963; R 2007; E 2014; E 2014) Particle-Size Analysis of Soils
ASTM D4318	(2010; E 2014) Liquid Limit, Plastic Limit, and Plasticity Index of Soils
ASTM D4643	(2017) Standard Test Method for Determination of Water Content of Soil and Rock by Microwave Oven Heating
ASTM D5084	(2016a) Standard Test Methods for Measurement of Hydraulic Conductivity of Saturated Porous Materials Using a Flexible Wall Permeameter
ASTM D6938	(2017) Standard Test Method for In-Place Density and Water Content of Soil and Soil-Aggregate by Nuclear Methods (Shallow Depth)

#### 1.2 DEFINITIONS

#### 1.2.1 Soil-Bentonite Fill

Soil-Bentonite Fill is a mixture of Select Fill or Embankment Fill, referred to as Base Soil, and dry powdered Bentonite materials uniformly blended and thoroughly compacted to produce a low permeability fill zone which meets the criteria set forth in the drawings and specifications. The Soil-Bentonite Fill shall contain Bentonite by dry weight of the Base Soil, as determined in the table below.

Table 1: Minimum Percent Bentonite of Soil-Bentonite Fill
Based on Percent Fines of the Base Soil

Base Soil Percent	Fines	Minimum	Percent	Bentonite*
<pre></pre>			15 12 9 6	

<sup>\*</sup>Tolerance of -1 percent allowed.

#### 1.2.2 Embankment Fill

Embankment Fill shall meet the properties defined in Section  $35\ 41\ 00$  EMBANKMENT CONSTRUCTION, subparagraph "Embankment Fill" of paragraph DEFINITIONS.

#### 1.2.3 Select Fill

Select Fill shall meet the properties defined in Section 35 41 00 EMBANKMENT CONSTRUCTION, subparagraph "Select Fill" of paragraph DEFINITIONS.

#### 1.2.4 Unsatisfactory Materials

Unsatisfactory Materials are defined in Section  $35\ 41\ 00$ , subparagraph "Unsatisfactory Materials" of paragraph DEFINITIONS.

# 1.2.5 Degree of Compaction

Degree of compaction required is expressed as a percentage of the maximum density obtained by the test procedure in ASTM D1557.

# 1.2.6 Bentonite

The Bentonite shall be a sodium cation base montmorillonite powder (Premium Grade Wyoming-type Bentonite) that conforms to the standards set forth in API Spec 13A, Section 9.

#### 1.2.7 Water

Clean, potable quality water, free of oils, acids, alkali, chlorides, organic matter, and other deleterious substances.

# 1.3 SUBMITTALS

Government approval is required for submittals with a "G" designation;

submittals not having a "G" designation are for information only. When used, a designation following the "G" designation identifies the office that will review the submittal for the Government. The following shall be submitted in accordance with Section 01 33 00 SUBMITTAL PROCEDURES:

# SD-02 Shop Drawings

Soil-Bentonite Fill Mix Design and Laboratory Test Results; G, DO

At least 30 days prior to Soil-Bentonite Fill construction, submit a report summarizing the procedures and results of the preconstruction Soil-Bentonite Fill mix design and tests, including sieve analysis, moisture-density relationship, liquid limit, plasticity index, and permeability.

# SD-03 Product Data

#### Equipment and Plans; G, DO

At least 30 days prior to plant assembly, submit details, plans (including the location of mixing plant and material storage areas), and data on the pugmill mixing plant, including the manufacturer's literature on feed equipment used to deliver the Bentonite and Base Soil, water controls, stockpiling procedures, and monitoring procedures. Prior to mobilization to the site, submit details on compaction equipment and placement procedures.

#### SD-06 Test Reports

# Soil-Bentonite Fill Mix Design and Laboratory Test Results; G, DO

At least 30 days prior to Soil-Bentonite Fill construction, submit a report summarizing the procedures and results of the preconstruction Soil-Bentonite Fill mix design and tests, including sieve analysis, moisture-density relationship, liquid-limit, plasticity index, and permeability.

# Calibration Curves; G, RO

Calibration curves and related test results, prior to using any device or equipment being calibrated.

# Soil Classification Test; G, DO

At least 30 days prior to delivery of any Contractor-furnished material to the site of the work, submit soil classification test results, moisture-density curves, gradation curves, and laboratory results of the required tests of the proposed material.

Field Density; G, RO Laboratory Density; G, RO Nuclear Density; G, RO

Use nuclear density testing equipment in accordance with ASTM D6938. In addition, the following condition shall apply:

a. Prior to using the nuclear density testing equipment on the site, submit to the Contracting Officer a certification that the operator has completed a training course approved by the nuclear

density testing equipment manufacturer, the most recent data sheet from the manufacturer's calibration, and a copy of the most recent statistical check of the standard count precision.

b. The nuclear density testing equipment shall be capable of extending a probe a minimum of 12 inches down into a hole.

#### SD-07 Certificates

# Certificates of Compliance; G, RO

The Contractor shall furnish to the Contracting Officer a certificate of compliance and a copy of the test reports from the Bentonite manufacturer for each lot of Bentonite shipped to the site stating that the Bentonite complies with all applicable standards. All Bentonite will be subject to inspection, sampling, and verification of quality by Contractor Quality Control testing and Government Quality Assurance testing. No Bentonite from the Bentonite manufacturer shall be used prior to acceptance by the Contracting Officer. Bentonite not meeting the specifications shall be promptly removed from the site and replaced with Bentonite conforming to specification requirements at the Contractor's expense. Bentonite shall be protected from moisture during transit and storage

#### PART 2 PRODUCTS

# 2.1 PLANT, EQUIPMENT, MACHINES, AND TOOLS

Plant, equipment, machines, and tools used in the work shall be subject to approval and shall be maintained in a satisfactory working condition at all times. The equipment shall have the capability of producing the required mix proportions, compaction, meeting grade controls, and thickness.

The central plant (pugmill operation) shall be capable of producing a uniform mixture of Soil-Bentonite Fill at the required Bentonite percentage and moisture content. Note that a volumetric mixer in lieu of a pugmill mixer will not be accepted under this contract. Soil and Bentonite shall be dry-mixed sufficiently to prevent Bentonite lumps from forming when water is added. Blending and mixing of Bentonite and Base Soil on a lift at the site (in-place mixing) will not be allowed.

# PART 3 EXECUTION

## 3.1 MIXING

Embankment Fill and dry powdered Bentonite shall be fed into a pugmill mixer at a uniform rate. The feed rate shall be predetermined by the Contractor prior to mixing Soil-Bentonite Fill for placement in the embankment. The mixture of Soil-Bentonite Fill produced by the central mixing process shall meet the requirements of Table 1. The mixing process shall be continuously monitored as part of the Contractor's quality control process. Hoppers delivering ingredients to the pugmill shall have features that prevent clogging of the hoppers.

Soil-Bentonite Fill not meeting the above prescribed tolerance will be rejected. Water shall be added to the mixture of Soil-Bentonite Fill such that the moisture content of the mixture produced by the central mixing

process will be in the range of -2 to +2 percent of the optimum moisture content as determined by ASTM D1557. The Embankment Fill, Bentonite, and Water shall be thoroughly mixed in the pugmill mixer such that each component of the mixture is uniformly distributed and the mixture is homogeneous.

#### 3.2 STOCKPILING MATERIALS

Base Soil, including approved material available from excavation and grading, shall be stockpiled in a manner that does not result in segregation of these materials (such as by conveyor or by chute) and at the locations designated. Before stockpiling of material, the storage sites shall be cleared, drained, and leveled. When Base Soil is provided from more than one source, material from each source shall meet the specified requirements and shall be stockpiled separately.

# 3.3 PREPARATION OF AREA TO BE FILLED

Adequate drainage shall be provided during the entire construction period to prevent water from collecting or ponding on the areas to be filled. No Soil-Bentonite Fill shall be placed in any area of the embankment until the foundation has been dewatered, compacted, and approved by the Contracting Officer. Line and grade stakes shall be located and placed by the Contractor as necessary for control. The area to be filled shall be prepared as described in Section 35 41 00.

#### 3.4 INSTALLATION

#### 3.4.1 Lift Thickness

The Soil-Bentonite Fill shall be placed in maximum 9-inch thick loose lifts.

# 3.4.2 Tolerances

A horizontal tolerance of 0 inches inside to 12 inches outside and a vertical tolerance of 3 inches above and 0 inches below the lines and grades shown will be allowed.

# 3.4.3 Compaction

After placing each lift of Soil-Bentonite Fill, compaction shall be started within a maximum of 30 minutes. Density of compacted Soil-Bentonite Fill shall be at least 95 percent of the Modified Proctor maximum dry density, determined in accordance with ASTM D1557. Loose lifts shall be uniformly and continuously compacted until the entire depth and width of the area are compacted to the density specified. Each pass of the compaction equipment shall overlap previous passes by at least one-half the width of the roller. A sheep's foot or pneumatic roller may be required. Alternate trips of the roller shall be slightly different lengths. Displacement of the mixture shall not occur due to the speed of the roller. The Soil-Bentonite Fill shall be placed such that the surface of the fill slopes downward away from the culvert barrels on a 6 horizontal to 1 vertical (6:1) slope; once fill reaches an elevation above the top of the barrels, the Soil-Bentonite Fill shall be brought to a level plane before receiving additional lifts. Each lift shall be compacted such that no slicken sides are found between compacted lifts. Scarify foundation and previously placed lifts to remove smooth surfaces prior to placement of additional lifts.

#### 3.5 MOISTURE CONTROL

The moisture content at the surface shall be maintained within the prescribed percent range of the optimum moisture content (discussed in MIXING) at all times throughout the compaction operations. If any in-place lift of Soil-Bentonite Fill is too wet prior to placing the next lift, the non-compliant material shall be allowed to dry to within moisture tolerance by disking, aerating, or other methods. If any in-place lift of Soil-Bentonite Fill is too dry prior to placing the next lift, the non-compliant material shall be loosened by scarifying or disking to depths directed by the Contracting Officer, dampened to within moisture tolerance, thoroughly mixed in-place to a homogenous mixture, and compacted in accordance with paragraph "Compaction". Alternately, the wet or dry material can be removed and replaced at the Contractor's expense.

# 3.6 FIELD QUALITY CONTROL

# 3.6.1 Test Reports

Field and laboratory tests shall be performed in sufficient numbers to assure that the specifications are being met. Testing shall be the responsibility of the Contractor and shall be performed by a USACE validated commercial laboratory. Work requiring testing will not be permitted until the testing laboratory has been inspected and approved. The Contracting Officer may specify the time and location of the test. Testing reports shall be furnished to the Contracting Officer within 24 hours of conclusion of all field and laboratory test, as well as on a monthly basis.

#### 3.6.2 Test Results

Results shall verify that materials comply with the specification. When a material source is changed, the new material shall be tested for compliance. When deficiencies are found, the initial analysis shall be repeated and the material already placed shall be retested to determine the extent of unacceptable material. All in-place material deemed unacceptable shall be replaced or repaired to conform to the contract requirements at no additional cost to the Government.

# 3.6.3 Permeability Test

One permeability test shall be performed in accordance with ASTM D5084 Method C for every 100 cubic yards of Soil Bentonite Fill produced, or at least three tests shall be performed overall, one from near the bottom of the core, one from near the center and one from near the top, whichever is more stringent. The test sample shall be collected in the field from the same batch as the GRAIN SIZE ANALYSIS mixed soil sample, prior to its compaction so that the samples are representative of each other.

# 3.6.4 Laboratory Density

One moisture density test shall be conducted in accordance with the procedure contained in  $\overline{ASTM}$  D1557. An additional test shall be performed for any change in Base Soil source or classification (as determined by  $\overline{ASTM}$  D2487), or at the direction of the Contracting Officer.

# 3.6.5 Field Density

Field in-place density tests shall be performed in accordance with ASTM D1556 and/or ASTM D6938. Perform not less than one in-place density test for each lift of Soil-Bentonite Fill per side of the culvert barrels\_and not less than one per lift at horizontal locations randomly staggered\_in the fill when fill has reached elevations above the barrels. When nuclear method is used for in-place density testing according to ASTM D6938, the first daily test and every tenth test thereafter or the first daily test and once every third lift thereafter, whichever is more frequent, for each material type shall include a sand cone correlation test in accordance with ASTM D1556. The sand cone test shall be performed adjacent to the location of the nuclear test, and shall include a nominal 6-inch diameter sand cone, and shall include a minimum wet soil weight of 6 pounds extracted from the hole. The density correlations shall be submitted with test results.

Each transmittal including density test data shall be provided electronically as an Excel Spreadsheet (.xls) file in addition to PDF format and must include a summary of all density correlations for the job neatly prepared on a summary sheet including at a minimum:

- a. Meter serial number and operator's initials
- b. Standard count for each test
- c. Description of material type, including USCS classification and color
- d. Probe depth
- e. Moisture content by each test method and the deviation
- f. Wet density by each test method and the deviation
- g. Detailed test location State plane coordinates  $(x \ and \ y)$ , elevation, and date
- h. Date of last calibration of meter including certification
- i. Corresponding Laboratory Density and Moisture Test
- j. Whether test passed or failed

#### 3.6.6 Moisture Content

Determination of water content shall be performed in accordance with ASTM D6938 when nuclear density test method is used. When nuclear method is used for in-place moisture content, the first and every tenth test thereafter for each material type shall include determination of moisture content based on ASTM D2216. ASTM D4643 may be used when rapid moisture content results are needed or as a substitute to ASTM D6938 or if it is determined that the latter is not reliable. All rapid results obtained by ASTM D4643 shall be confirmed by a test on a duplicate sample performed in accordance with ASTM D2216. In the event of significant disagreement (more than 2%) between the ASTM D6938 and ASTM D2216 tests, ASTM D2216 shall govern and ASTM D6938 shall no longer be used until calibration tests show agreement between methods. In the event that ASTM D6938 cannot be calibrated to provide accurate results, ASTM D4643 and ASTM D2216 shall be used exclusively to determine moisture. One (1) moisture content test will be performed each time a density test is performed on any given lift.

# 3.6.7 Grain Size Analysis

Grain size analysis tests shall be performed in accordance with ASTM D422. One series of tests shall be conducted at the beginning of each work shift where Soil-Bentonite Fill is to be produced. A series of tests shall include a grain size analysis on representative samples of the stockpiled

Base Soil and one grain size analysis on representative samples of the mixed Soil-Bentonite Fill, as placed.

# 3.6.8 Liquid Limit and Plasticity Index

One liquid limit and plasticity index shall be performed on the Soil-Bentonite Fill for each grain size analysis conducted. Liquid limit and plasticity index shall be in accordance with ASTM D4318.

# 3.6.9 Additional Testing

The Contracting Officer may request additional tests if there is reason to doubt the adequacy of the compaction, special compaction procedures are being used, materials change, if the Contracting Officer determines that the Contractor's testing is inadequate, or the Contractor is concentrating backfill and fill operations in a relatively small area.

# 3.6.10 Testing By The Government

During the life of this contract, the Government or its subcontracted lab will perform quality assurance tests. The Contractor shall make materials available for testing and expose materials for testing at the request of the Contracting Officer for the duration of this contract. The Contractor will provide a minimum of 72 hour notice prior to conducting sample collection at material provider site to allow government the ability to schedule resources for any split sampling during the duration of the contract.

# 3.6.11 Reporting

On a daily basis, furnish the inspection records and all material testing results, the quantity of fill placed, as well as the records of corrective action taken, in accordance with Section 01 45 04 CONTRACTOR QUALITY CONTROL.

-- End of Section --

CONTRACT NO. W912EP-18-R-0013

TITLE AND LOCATION CONTRACTOR Herbert Hoover Dike Rehabilitation. Structure Replacements CONTRACTOR: CONTRACTOR APPROVING AUTHORITY G SCHEDULE DATES ACTION 0 ٧ С Т R L A N S M Α 0 S R A C T A C T S Α PEC A R F 1 Т Ε 0 DATE FWD 0 MAILED TO APPR Т Α С Ν Ν TO CONTR/ G # Α R AUTH/ S R Т Ε E DESCRIPTION Α ٧ APPROVAL MATERIAL 0 DATE DATE RCD DATE FWD DATE RCD 0 DATE DATE RCD TO OTHER FROM OTH REVIEWER Ν Ν С Ρ 0 W NEEDED **NEEDED** D OF FROM OF FRM APPR 0 ACTION Ε ACTION 0 ITEM SUBMITTED Н Ν R SUBMIT BY Ε CONTR AUTH REMARKS (b) (d) (f) (j) (a) (c) (e) (g) (h) (i) (k) (I) (m) (n) (o) (p) (q) (r) SD-02 Shop Drawings 01 11 00 Bypass of Water Plan G DO Traffic Plan G RO Temporary Safety Barrier Plan 1.5.5 G RO Temporary Turnouts Design Plan 1.3.3 G RO 01 22 00 SD-01 Preconstruction Submittals Schedule of Values G RO SD-07 Certificates Request for Progress Payment 3.1.1 G RO Request for Progress Payment 3.1.1 G RO 01 30 00 SD-01 Preconstruction Submittals List of Subcontractors SD-07 Certificates Signature Authority Drug-Free Work Place Record 01 32 01 SD-01 Preconstruction Submittals 1.3 G RO **Project Scheduler Qualifications** Preliminary Project Schedule 3.4.1 G RO Initial Project Schedule 3.4.2 G RO 3.6.2 G RO Periodic Schedule Update 01 33 00 SD-01 Preconstruction Submittals Export File G RO 01 35 25 SD-01 Preconstruction Submittals **Dive Operations Plan** G RO SD-07 Certificates G RO **Dive Personnel Qualifications** 

CONTRACT NO. W912EP-18-R-0013

TITLE AND LOCATION CONTRACTOR Herbert Hoover Dike Rehabilitation, Structure Replacements CONTRACTOR: CONTRACTOR APPROVING AUTHORITY G SCHEDULE DATES ACTION 0 ٧ С Т R L A N S M Α 0 S R A C T A C T S Α P E C A R F 1 Т Ε 0 DATE FWD 0 MAILED TO APPR Т Α С Ν Ν TO CONTR/ G # Α R AUTH/ S R Т Ε E DESCRIPTION Α ٧ APPROVAL MATERIAL 0 DATE DATE RCD DATE FWD DATE RCD 0 DATE DATE RCD TO OTHER FROM OTH REVIEWER REVIEWER Ν Ν С Ρ 0 W NEEDED **NEEDED** D OF FROM OF FRM APPR 0 ACTION Ε ACTION 0 ITEM SUBMITTED Н Ν R SUBMIT BY Ε CONTR AUTH REMARKS (b) (d) (e) (f) (j) (a) (c) (g) (h) (i) (k) (l) (m) (n) (o) (p) (q) (r) 01 35 26 SD-01 Preconstruction Submittals Accident Prevention Plan (APP) 1.6 G DO Activity Hazard Analysis (AHA) 1.7 G DO SD-02 Shop Drawings Crane Critical Lift Plan 1.6.1 G DO Standard Lift Plan (SLP) 1.6.1 G RO SD-06 Test Reports Notifications and Reports 1.11 **Accident Reports** 1.11.1 Crane Reports 1.11.2 SD-07 Certificates Confined Space Entry Permit 1.8 Hot work permit 1.8 License Certificates 1.13 1.5.2.3 G DO **Crane Operators** Certificate of Compliance 1.11.3 01 45 04 SD-01 Preconstruction Submittals Contractor Quality Control (CQC) 3.2 G RO Plan Initial Work Schedule G RO SD-07 Certificates 3.7.2 G RO **Laboratory Qualifications** Letter of Authority 3.2.2 SD-01 Preconstruction Submittals 01 50 02 Mobilization/Demobilization Plan 3.8 Security Plan

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			Transfer Prevention Plan															
			Environmental Protection Plan		G DO													
			SD-07 Certificates															
			Bird Monitoring Qualifications		G DO													
			Eastern Indigo Snake Observer		G DO													
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Formwork Not Supporting Weight 3.4.1

of Concrete SD-02 Shop Drawings

Waterstops

SD-03 Product Data

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